





# PANAGIA MELANDRINA CHURCH

Structural Assessment And Design Of Emergency Repairs And Supporting Measures

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# BIENNIAL OF ARCHITECTURAL AND URBAN RESTORATION,

15-16-17 APRIL 2018

Famagusta Gate - Nicosia







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**Architects** 

Marilia Christodoulou Architect

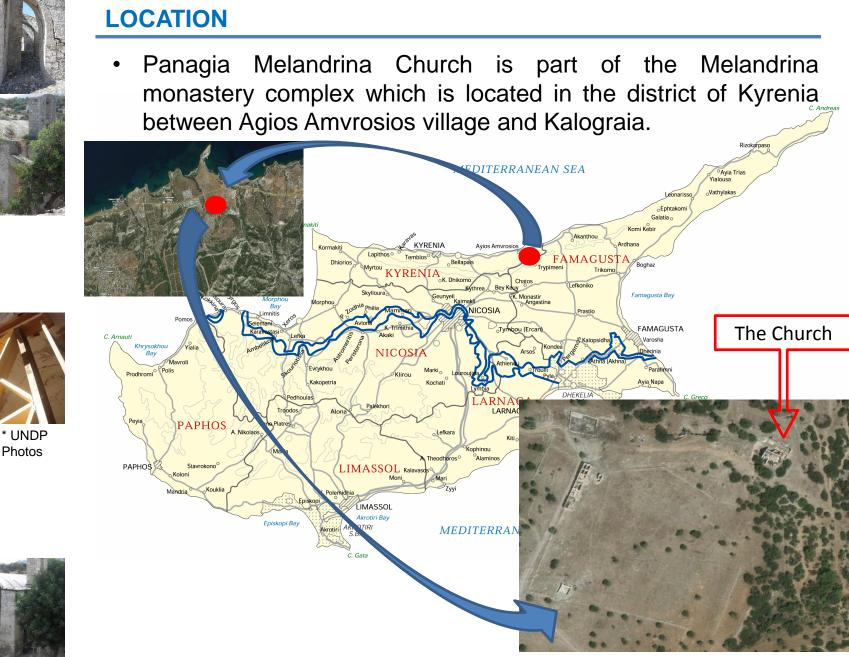
## Zoi Lavrentaki

Architect

\* All photos and drawings are part of UNDP's Study
\*\* The contents of this presentation are abstracts

from a EU funded and UNDP commissioned study

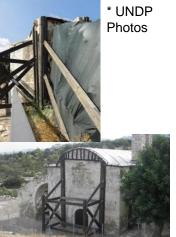






# OBJECTIVE

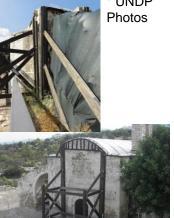
- Abandonment & lack of regular maintenance work and protection over the years are the main problems of the site.
- In 2011, a number of site inspections took place to observe & record the current condition of the structure and the site.
- Our team was responsible to investigate the structure's pathologies, assess the safety of the church and design emergency repair & supporting measures that would prevent the structure from further damage without distorting its architectural and monumental character.
- One of the most challenging parts of the project was the fact that all proposed interventions should be reversible and help / facilitate the execution of any future restoration works.



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\* UNDP Photos

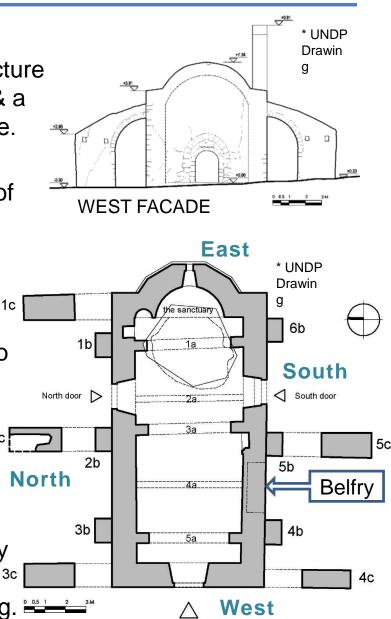


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# THE STRUCTURE

- Late 14<sup>th</sup> century.
- Single-naved, barrel-vault structure with an apse at the sanctuary & a small belfry on the south façade.
- Three leaf masonry walls with approximate overall thickness of 120cm.
- Mainly lime mortar Cement • mortar plaster in the vault and sanctuary.
- 5 arched masonry beams [1a to 5a].
- 3 steel tie rods -1 is cut.
- 6 external masonry piers buttresses [1b to 6b].
- 5 masonry counterforts added around 1731 [1c to 5c] – Simply attached to the structure / Sculptured frame & rubble filling.





Photos

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#### **PHOTOS – Before Interventions**



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Photo Archives-Department of Antiquities



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## **CONDITION ASSESSMENT & PATHOLOGY**

- Objective involved the assessment of both the **body of the Church** itself and the added **Counterforts**.
- Very bad structural & general condition Extensive damages and deformations on the load bearing systems due to:
  - earthquake activity & ground movements
  - lack of regular maintenance / restoration / renovation.
- Very poor or no joining & adhesion of stones leads each structural component to act individually according to its own mechanical / dynamic characteristics – Walls with no monolithic behaviour / Roof with no diaphragmatic action.
- Extensive severe cracks on the load bearing masonry of the Church – Mostly out-of-plane cracking patterns.



\* UNDP Photo



\* UNDP Photo





\* UNDP Photo

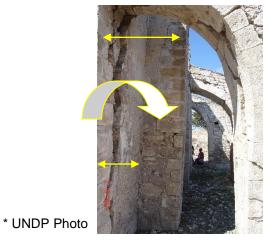
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## **CONDITION ASSESSMENT & PATHOLOGY**

Extensive severe cracks on the load bearing masonry of the Church – Large dislocations & torsional displacements on E side.





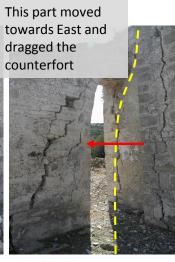
\* UNDP Photo

Extensive severe cracks on all counterforts and partial collapse of one of them.



\* UNDP Photo

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The dragging caused the counterfort to split [max 120mm].



\* UNDP Photo





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# **CONDITION ASSESSMENT & PATHOLOGY**

 Extensive damages on all arched masonry beams & total collapse of one of them.



\* UNDP Photo



\* UNDP Photo

• Partial collapse of the roof & extensive cracking on the rest of it.



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# **CONDITION ASSESSMENT & PATHOLOGY**

• Poor or no joining plaster, mortar & pointing in masonry walls.



\* UNDP Photo



<sup>\*</sup> UNDP Photo

• Uncontrolled plantation / vegetation inside the body of the walls, roof and counterforts.



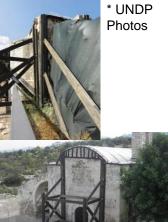
\* UNDP Photo





\* UNDP Photo





#### **ENGINEERING JUDGEMENT & MODELING**

- Barrel Vault behavior:
  - Figure 1:

Integrity is ensured when internal loads [red arrows] are in equilibrium with external forces [green arrows].

- Figure 2 & 3:

Integrity is breached when equilibrium is disturbed either by failure of the supports or instability of the lateral elevation.

In both cases, hinges and hence mechanisms are developing that lead the system out of balance.

 In our case, absence of monolithic action, no diaphragm & weak buttresses due to poor binding mortar led to system instability

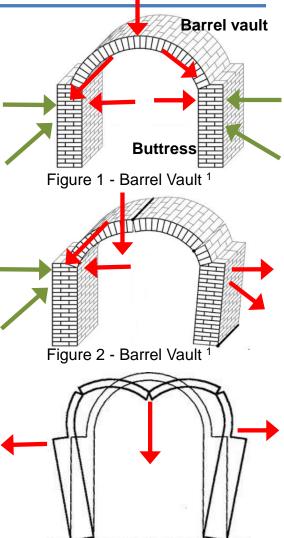


Figure 3 - Barrel Vault<sup>2</sup>

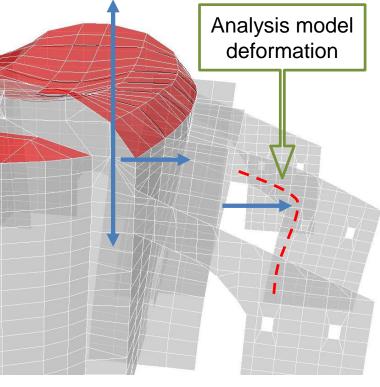
<sup>1</sup> ASCE Library Journal of Structural Engineering | Vol 143, No 6

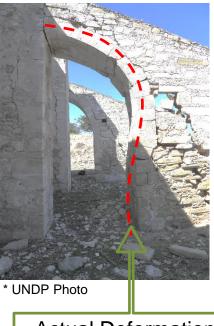
<sup>2</sup> <u>Statics of Historic Masonry Constructions</u> pp 273-289 / Part of the <u>Springer Series in Solid and Structural</u> <u>Mechanics</u> book series



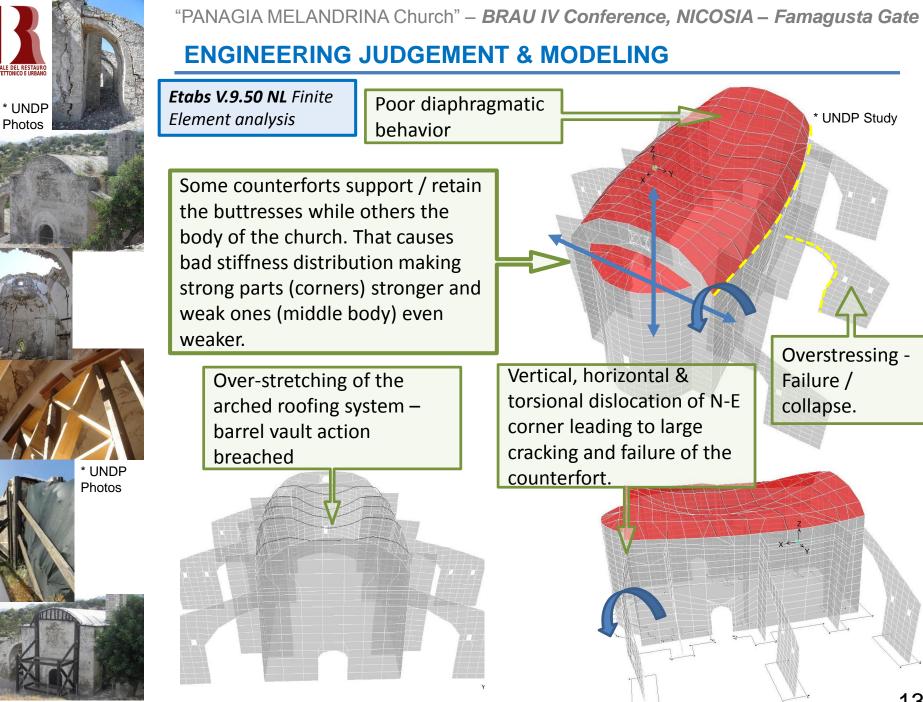
## **ENGINEERING JUDGEMENT & MODELING**

- Counterforts behavior:
- Simply attached to the body of the Church No participation in gravity loads.
- All diagonal cracks are due to horizontal "pushing" coming from the main structure's vertical deformation.





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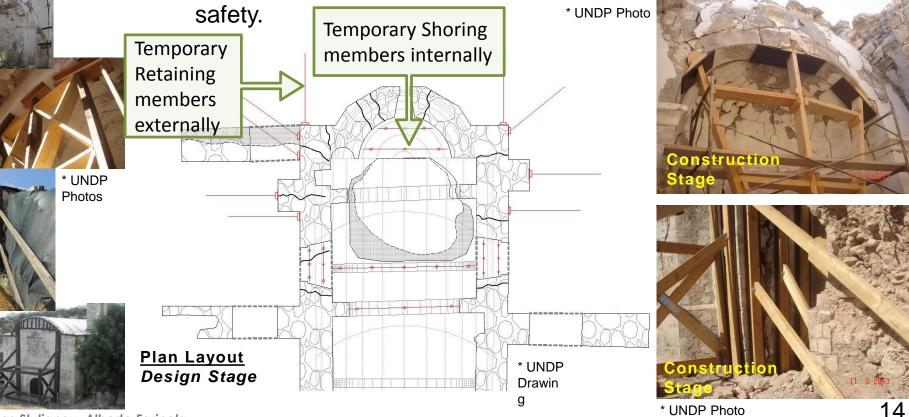




# **INTERVENTIONS**

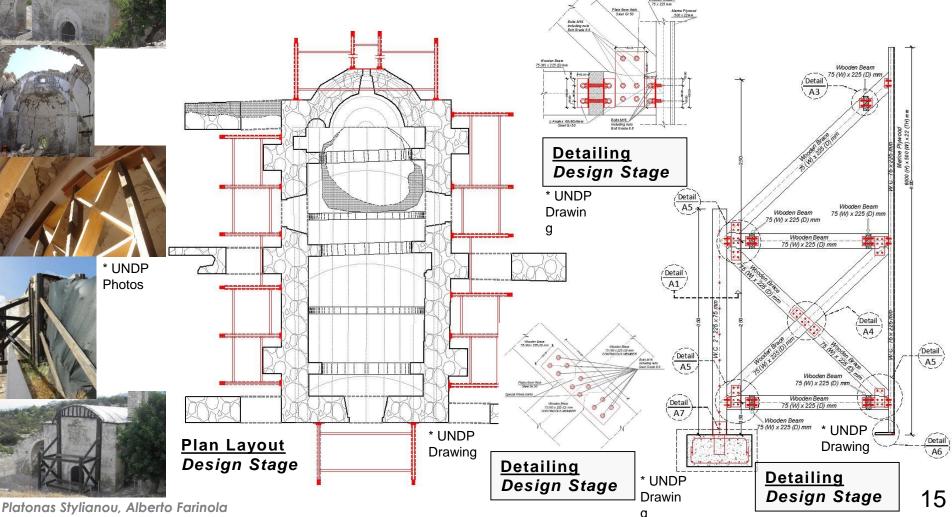
# **Temporary Retaining & Shoring system**

- Prior to the commencement of any construction work, temporary timber shoring & retaining members were installed internally and externally at the more stressed & damaged upper half of the structure towards East.
- Some of the external retaining frames remained for additional



# **INTERVENTIONS**

\* UNDP Photos • External Timber Truss Retaining system - To prevent the structure from further deformation, displacement and cracking, vertical timber trusses are installed at selected places all around the structure.





# **INTERVENTIONS**

**External Timber Truss Retaining system** - Photos







\* UNDP Photo

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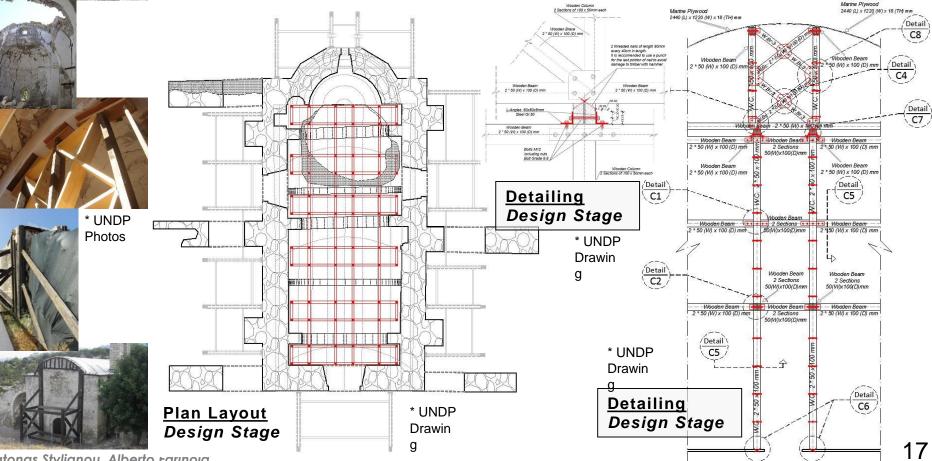
\* UNDP Photo

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# **INTERVENTIONS**

Internal Timber Truss Shoring system - A timber truss system is installed in the interior of the Church to support the damaged vaulted roof preventing further collapsing. This system is designed and constructed so as to not disturb any future renovation works but even more, facilitate / help their execution.



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\* UNDP

Photos



## **INTERVENTIONS**

## Internal Timber Truss Shoring system - Photos



\* UNDP Photo



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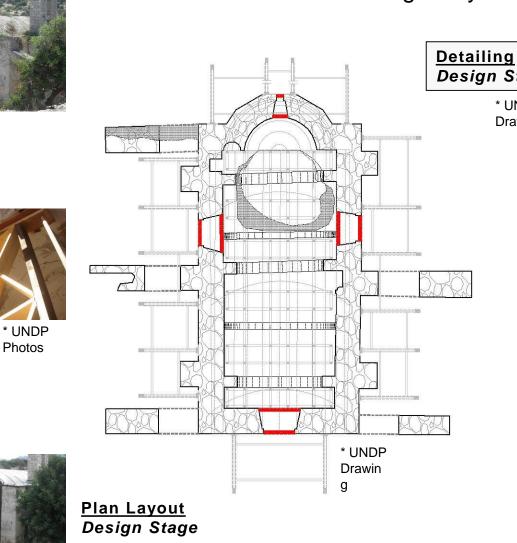
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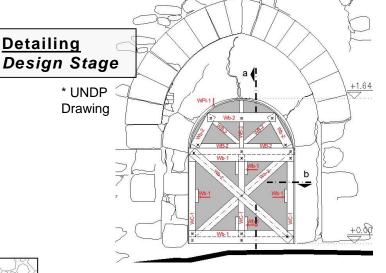


\* UNDP Photo

## **INTERVENTIONS**

Bracing of all openings - Bracing of all openings is done to reestablish the wall's homogeneity.







\* UNDP Photo

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Photos

\* UNDP Photos



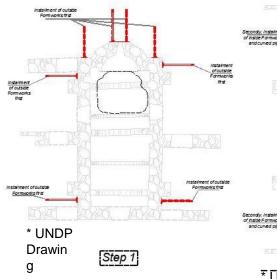


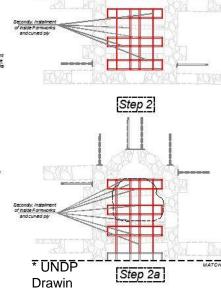
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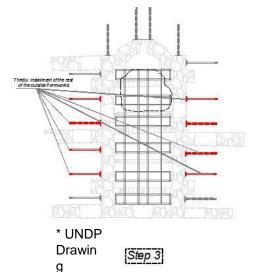
MATCHPOINT

## **INTERVENTIONS**

# Construction Sequence





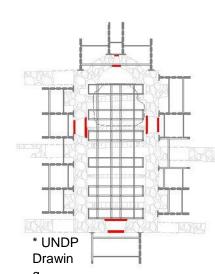




installed len beams connecting

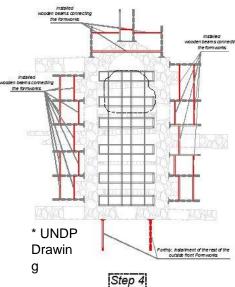
the formworks

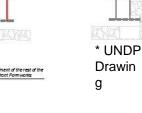
Step 5



Step 6: Bracing of openings

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## **INTERVENTIONS**

• **Grouting of all large cracking** - Grouting injections with limebased mortar inside the masonry wall in all large cracks in order to re-establish the wall's homogeneity.



\* UNDP Photo

\* UNDP Photo

Covering & protection of the roof – Curved Marine Plywood panels & heavy duty carbonate type are installed to cover & protect interior from rain, water, birds or other possible enemies causing further damage.



\* UNDP Photo

 Removing plantation / vegetation with pesticide - Growing plantation causes further damage in the body of the walls.
 Plantation should be removed before and after the completion of emergency measures



\* UNDP Photos



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# **CONCLUSION**

The measures taken are only to support and "relief" the structure in some extent and protect it from further damages until a full restoration scheme takes place. Regular maintenance works will help the structure withstand ageing and environmental impact.

Some next steps that would provide additional information and help future designs would be:

- Measuring and monitoring of the cracks ٠
- Measuring and monitoring of the relevant movements and verticality of the structure
- Full topographical survey of both the site and the monument •
- Further historical analysis and data recording of the church ٠
- Complete geological / geotechnical investigation.